

# **Declaration of Performance**

## 2323-CPR-0058

**1. Unique identification code of the product-type:** Mungo metal Anchor MEA for multiple use for nonstructural applications in concrete

2. Manufacturer: Mungo Befestigungstechnik AG, Bornfeldstrasse 2, CH-4600 Olten/Switzerland

#### 3. System/s of AVCP: System 2+

#### 4. Intended use or use/es:

Product	Intended use
Anchor for multiple use for non-structural	The anchor is to be used for static or quasi-static loading in
application in non-cracked and cracked	reinforced or unreinforced normal weight concrete of strength
concrete	classes C20/25 to C50/60 according to EN 206-1

 5. European Assessment Document: ETAG 001 Part 6, April 2013, used as EAD European Technical Assessment: ETA-18/0269 of 03.04.2018 Technical Assessment Body: ETA-Denmark A/S Notified body/ies: No 305/2011 (Construction Product Regulation)

#### 6. Declared performance:

#### Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for all load directions	See appendix, especially Annex C1 to C3
Edge distances and spacing	See appendix, especially Annex B2

#### Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	See appendix, especially Annex C5
Resistance to fire	See appendix, especially Annex C4

The performance of the product identified above is in conformity with the set of declared performance/s. This declaration of performance is issued, in accordance with Regulation (EU) No 305/2011, under the sole responsibility of the manufacturer identified above.

Singed for and on behalf of the manufacturer by:

Robert Klemencic Dipl.-Ing. Head of Engineering

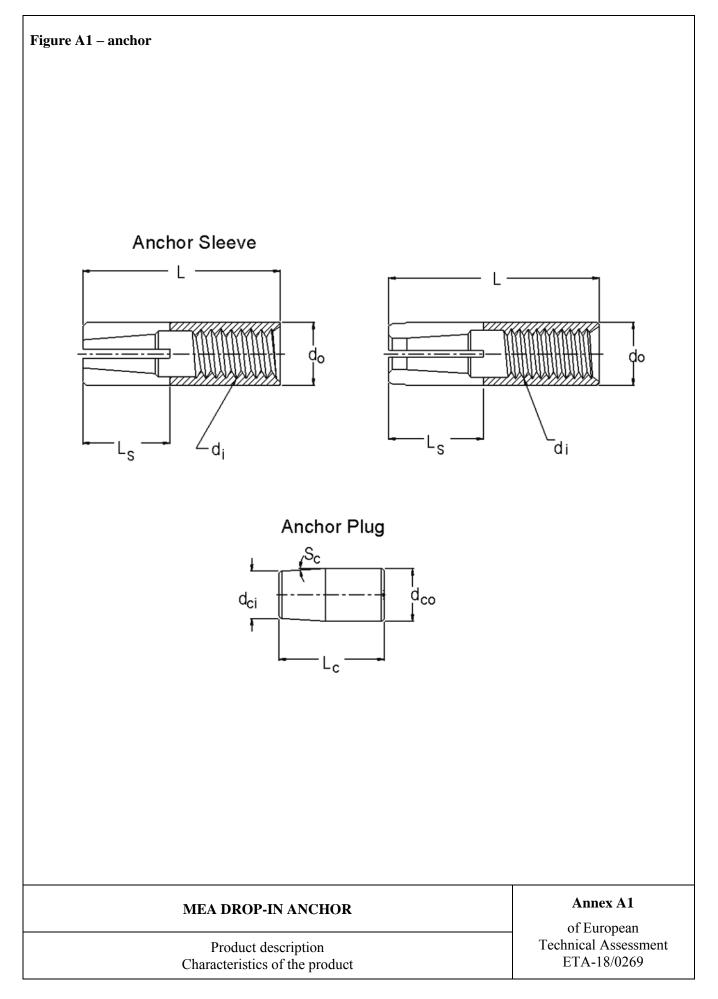


Olten, 18.11.2019

This DoP Has been prepared in different languages. In case there is a dispute on the interpretation the English version shall always prevail.

The Appendix includes voluntary and complementary information in English language exceeding the (language as neutrally specified) legal requirements.

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	Diameter inside	Length	spread outside		Length of cone	Diameter cone outside	Diameter cone inside	square
	$\mathbf{d}_{\mathbf{i}}$	L	$\mathbf{L}_{\mathbf{s}}$	do	Lc	d <sub>co</sub>	$\mathbf{d}_{\mathrm{ci}}$	Sc
	[ <b>mm</b> ]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[°]
	M6	24.90	11.60	7.94	10.00	5.05	3.95	5.00
	IVIO	$\pm 0.30$	$\pm 0.60$	$\pm 0.07$	$\pm 0.20$	$\pm 0.05$	$\pm 0.05$	$\pm 0.50$
	MO	29.90	13.80	9.94	11.90	6.25	4.50	6.00
	M8	$\pm 0.30$	$\pm 0.60$	$\pm 0.07$	$\pm 0.30$	$\pm 0.25$	$\pm 0.25$	$\pm 2.00$
	M10	39.60	18.35	11.94	15.70	7.85	6.30	6.00
	M10	$\pm 0.40$	$\pm 0.75$	$\pm 0.07$	$\pm 0.30$	$\pm 0.25$	$\pm 0.30$	$\pm 2.00$
Ī	M12	50.50	22.75	14.94	20.70	10.05	8.50	4.00
	M12	$\pm 0.50$	$\pm 0.75$	$\pm 0.07$	$\pm 0.30$	$\pm 0.25$	$\pm 0.30$	$\pm 2.00$
Ī	M16	65.00	29.35	19.80	28.10	13.85	11.70	3.50
	M16	$\pm 0.50$	$\pm 0.75$	$\pm 0.20$	$\pm 0.30$	$\pm 0.25$	$\pm 0.30$	$\pm 2.00$

Diameter inside	Length	Length of spread	Diameter outside	Length of cone	Diameter cone outside	Diameter cone inside	square
di	L	Ls	do	Lc	d <sub>co</sub>	dci	Sc
[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[°]
M8x25	24.90	11.15	10.00	8.15	6.40	5.40	4.5
IVIOX23	$\pm 0.30$	$\pm 0.60$	- 0.13	$\pm 0.20$	$\pm 0.05$	$\pm 0.05$	± 0.5
M10x25	24.60	11.60	12.00	8.80	8.30	7.50	3.5
W110X23	$\pm 0.40$	$\pm 0.60$	- 0.13	$\pm 0.20$	$\pm 0.05$	$\pm 0.05$	± 0.5
M10x30	29.60	15.00	12.00	13.60	7.85	6.70	3.5
WITUX50	$\pm 0.40$	$\pm 0.60$	- 0.13	$\pm 0.20$	$\pm 0.05$	$\pm 0.05$	± 0.5
M12x25	24.60	11.20	15.00	10.45	9.80	8.60	7.0
IVI 1 2X23	$\pm 0.40$	$\pm 0.60$	- 0.13	$\pm 0.20$	$\pm 0.05$	$\pm 0.05$	± 0.5

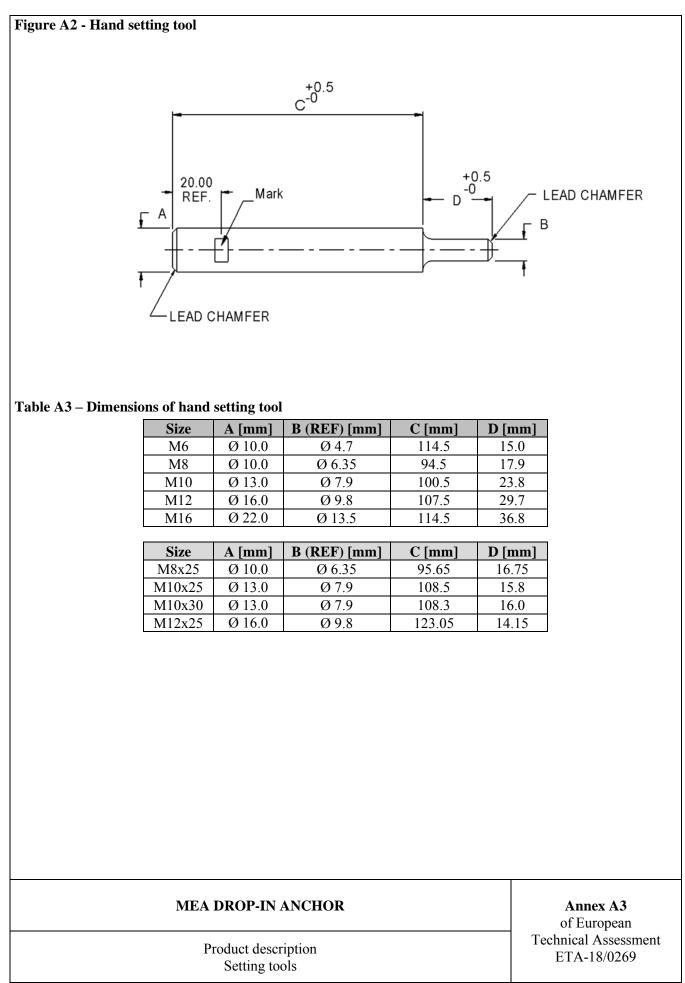
## Table A2. Materials

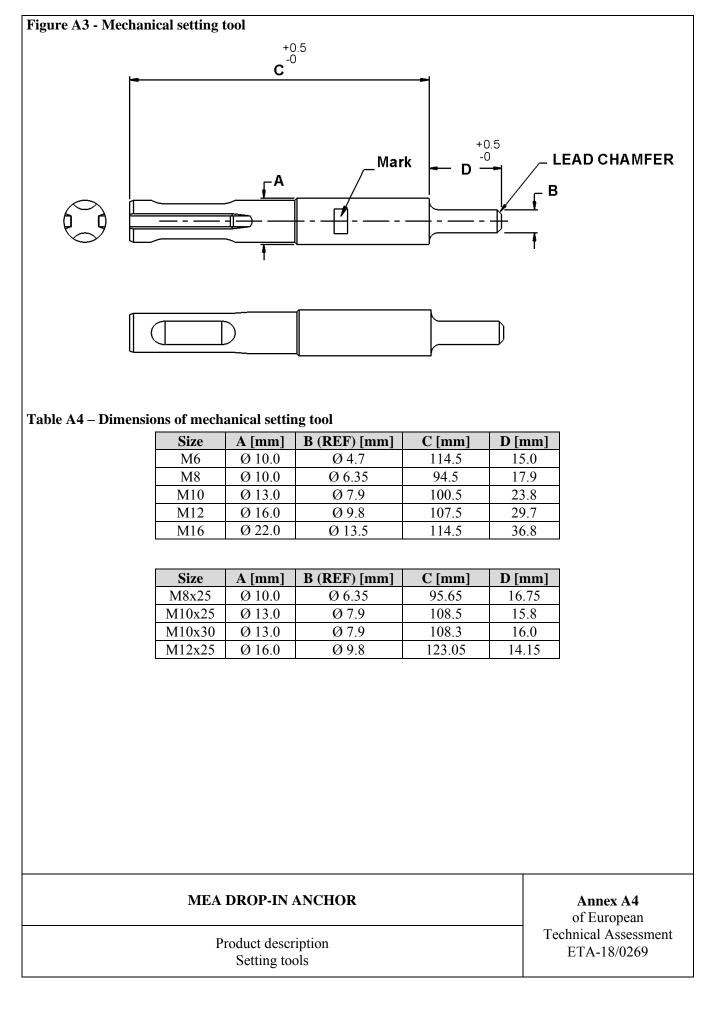
Member	Material
Sleeve	Coldformed steel grade C8C in accordance with table 2 in EN 10263-2 or coldformed steel grade 1008 in accordance with table 3 in ASTM A510. Galvanized
Plug	Coldformed steel grade C8C in accordance with table 2 in EN 10263-2 or coldformed steel grade 1008 in accordance with table 3 in ASTM A510. Galvanized

## MEA DROP-IN ANCHOR

Product description Materials Annex A2

of European Technical Assessment ETA-18/0269





Use:

The anchors are intended to be used for anchorages for which requirements for mechanical resistance and stability and safety in use in the sense of the Basic Requirements 1 and 4 of Regulation 305/2011 (EU) shall be fulfilled and failure of anchorages made with these products would compromise the stability of the works, cause risk to human life and/or lead to considerable economic consequences.

## Anchors subject to:

- Multiple use for non-structural applications.
- Static and quasi-static loads.

### **Base materials:**

- Reinforced or unreinforced normal weight concrete of strength class C20/25 at minimum to C50/60 at maximum according to EN 206-1.
- Cracked and non-cracked concrete

### Use conditions (Environmental conditions):

- Internal dry conditions

### Installation:

- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- Anchor installation in accordance with the manufacturer's specifications and drawings and using the appropriate tools.
- Check before placing the anchor to ensure that the strength class of the concrete, in which the anchor is to be placed, is identical with the values which the characteristic loads apply.
- Check of concrete being well compacted, e.g. without significant voids.
- Edge distances and spacings not less than the specified values without minus tolerances.
- Positioning of the drill holes without damaging the reinforcement.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole or smaller distance if the aborted drill hole is filled with high strength mortar and if under shear or oblique tension load it is not the direction of load application.
- Hole shall be clear.
- Anchor installation such that the effective anchorage depth is complied with; the compliance is ensured if the thickness of the fixture is not larger than the maximum values given in Annex B2.
- Anchor expansion by impact on the wedge of the anchor; the anchor is properly set if the wedge is fully dropped in.

#### **Proposed design methods:**

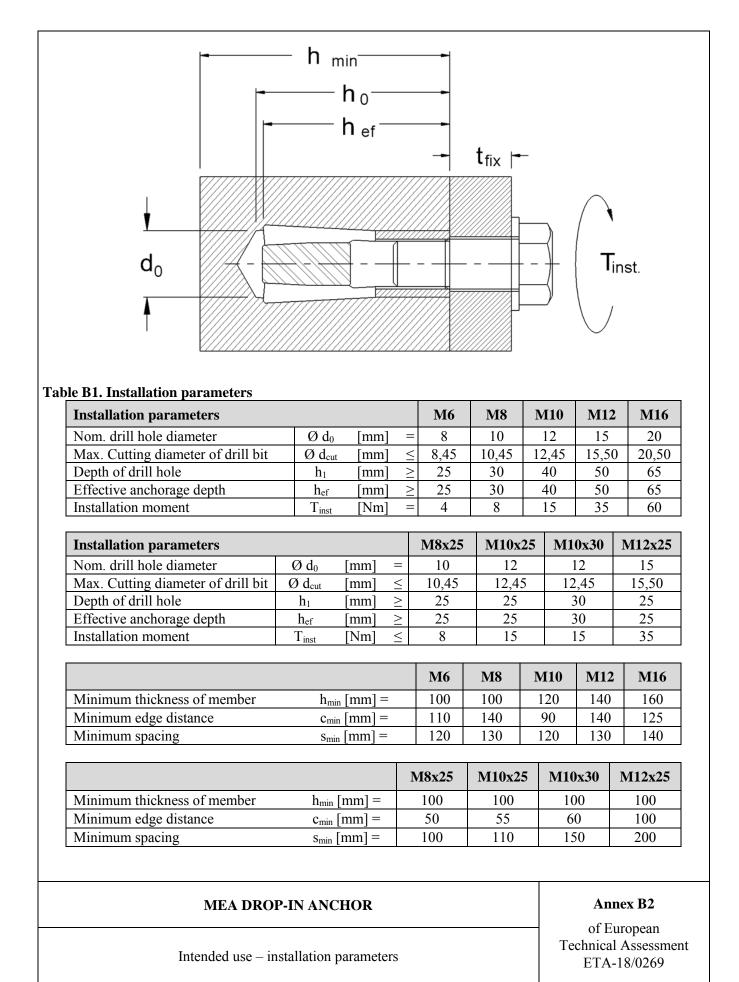
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the loads to be transmitted. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages under static and quasi-static loads are designed in accordance with EN 1992-4.
- Fasteners are only to be used for multiple use for non-structural applications acc. to ETAG 001, Part 6, Edition August 2010.

## MEA DROP-IN ANCHOR

Annex B1

Intended use - Specification

of European Technical Assessment ETA-18/0269



### Table C1: Design method C, characteristic tension load values

			M6	M8	M10	M12	M16
Steel failure							
Resistance to steel failure	N <sub>Rk,s</sub>	[kN]	9,92	14,62	15,24	30,92	49,90
Partial safety factor under tension load	γ <sub>Ms</sub>	[-]	1,40	1,40	1,40	1,40	1,40
Pull-out failure							
Resistance to pull-out failure in cracked concrete C20/25	N <sub>Rk,cr</sub>	[kN]	2,0	2,0	4,0	3,5	6,0
Increase factors for non-cracked concrete	$\Psi_{c}$	[-]	1,35	1,25	1,47	1,55	1,55
Concrete cone failure							
Effective embedment depth	h <sub>ef</sub>	[mm]	25	30	40	50	65
Edge distance	C <sub>cr,N</sub>	[mm]	1,5xh <sub>ef</sub>				
Spacing	S <sub>cr,N</sub>	[mm]	3xh <sub>ef</sub>	$3xh_{ef}$	$3xh_{ef}$	3xh <sub>ef</sub>	$3 x h_{ef}$
Robustness							
Installation safety factor	γinst	[-]	1,2	1,2	1,2	1,4	1,0
Minimum edge distance and spacing							
Minimum edge distance	c <sub>min</sub>	[mm]	110	140	90	140	125
Minimum spacing distance	Smin	[mm]	120	130	120	130	140
Min. thickness of the concrete member	<b>h</b> <sub>min</sub>	[mm]	100	100	120	140	160
Edge distance to prevent splitting under							
	N <sup>0</sup> <sub>Rk,sp</sub>	[kN]	2,0	2,0	4,0	3,5	6,0
Appropriate edge distance	Ccr,sp	[mm]	110	140	90	140	125
Displacements under static and quasi-							
static loading							
Short time tension displacement	$\delta_{N0}$	[mm]	0,10	0,35	0,09	0,08	0,32
Long-time tension displacement	$\delta_{N^\infty}$	[mm]	-	-	0,09	-	-

## MEA DROP-IN ANCHOR

Performance for static and quasi-static loads: Resistances

Annex C1 of European Technical Assessment ETA-18/0269

## Table C1A: Design method C, characteristic tension load values

			M8x25	M10x25	M10x30	M12x25
Steel failure			_			
Resistance to steel failure	N <sub>Rk,s</sub>	[kN]	14,13	15,24	15,24	30,92
Partial safety factor under tension load	γms	[-]	1,40	1,40	1,40	1,40
Pull-out failure						
Resistance to pull-out failure in cracked concrete C20/25	N <sub>Rk,cr</sub>	[kN]	0.9	1.5	3.0	2.0
Increase factors for non-cracked concrete	Ψc	[-]	1.34	1.45	1.19	1.45
Concrete cone failure						
Effective embedment depth	h <sub>ef</sub>	[mm]	25	25	30	25
Edge distance	c <sub>cr,N</sub>	[mm]	1,5xh <sub>ef</sub>	1,5xh <sub>ef</sub>	1,5xh <sub>ef</sub>	1,5xh <sub>ef</sub>
Spacing	S <sub>cr,N</sub>	[mm]	3xh <sub>ef</sub>	3xh <sub>ef</sub>	3xh <sub>ef</sub>	3xh <sub>ef</sub>
Robustness						
Installation safety factor	Yinst	[-]	1.4	1.2	1.4	1.4
Minimum edge distance and spacing						
Minimum edge distance	C <sub>min</sub>	[mm]	50	55	60	100
Minimum spacing distance	S <sub>min</sub>	[mm]	100	110	150	200
Min. thickness of the concrete member	$\mathbf{h}_{\min}$	[mm]	100	100	100	100
Edge distance to prevent splitting under load			1		1	
	N <sup>0</sup> Rk,sp	[kN]	0.9	1.5	2.0	2.0
Appropriate edge distance	C <sub>cr,sp</sub>	[mm]	60	75	90	100
Displacements under static and quasi-static						
loading						
Short time tension displacement	$\delta_{N0}$	[mm]	0.10	0.14	0.28	0.31
Long-time tension displacement	$\delta_{N^\infty}$	[mm]	-	-	0.40	-

## **MEA DROP-IN ANCHOR**

Performance for static and quasi-static loads: Resistances

Annex C2 of European Technical Assessment ETA-18/0269

## Table C2: Design method C, Characteristic shear load values

			M6	M8	M10	M12	M16
Resistance to steel failure under shear load							
Resistance to shear load without lever arm	V <sup>0</sup> <sub>Rk,s</sub>	[kN]	2,5	5,0	6,0	7,5	16,0
Resistance to shear load with lever arm	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	18,5	33,4	46,5	114	245
Displacements under static and quasi-static la	oading						
Short time shear displacement	δνο	[mm]	0,51	0,61	0,45	0,23	0,38
Long-time shear displacement	$\delta_{V^\infty}$	[mm]	0,77	0,92	0,68	0,35	0,57

			M8x25	M10x25	M10x30	M12x25
Resistance to steel failure under shear load						
Resistance to shear load without lever arm	V <sup>0</sup> <sub>Rk,s</sub>	[kN]	4.0	7,0	6.5	5.0
Resistance to shear load with lever arm	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	34.7	46.5	46.5	114.0
Displacements under static and quasi-static lo	oading					
Short time shear displacement	$\delta_{V0}$	[mm]	0.33	0.76	1.37	0.05
Long-time shear displacement	$\delta_{V^\infty}$	[mm]	0.50	1.14	2.06	0.08

#### **MEA DROP-IN ANCHOR**

Performance for static and quasi-static loads: Resistances and Displacements

Annex C3 of European Technical Assessment ETA-18/0269

	M16×65	2 14	1.60	1.39	1.07		000	00'0	2,40		6,13	4,91		260					M16×65	2,14	1,60	1,39	1,07	10.49	7,87	6,82	c <u>7</u> ,c	2,00	10 26	12,20	9,81		<sub>Rk,c</sub> (≤ R90) V <sup>0</sup> <sub>Rk,c,fi</sub> =		
	M12x50	1 24	0.93	0.81	0.62		010	200,4	2,00		3,18	2,55		200					M12x50	1,24	0,93	0,67	70'0	4.59	3,44	2,98	67'7	1,00	2.40	0,10	2.55	) I	0,25 x V <sup>0</sup> <sub>Rk,c</sub> (≤ R		
120	M10×40	0.50	0.43	0.33	0.27		4 7E	2.1	1,40		1,82	1,46		160		J.	s 300	20	M10x40	0,50	0,43	0,33	0,27	1.53	1,32	1,02	0,81	1,00	1 0.7	1,02	1,46	2	11		
to EOTA TR020	M10x3(	0.50	0.43	0.33	0.27		00 1	00,-	1,10		0,89	0,71	ef	150	L	one side: 2*h	e than 1 side: 3	to EOTA TR020	M10×30	0,50	0,43	0,33	17'0	1.53	1,32	1,02	0,81	1,00	08.0	60'0	0.71		termined by: \		
in accordance	M10x25	0.50	0.43	0.33	0.27		00.0	00,0	0,30		0,56	0,45	4*h <sub>ef</sub>	110	2*h <sub>ef</sub>	Fire attack from one side: 2*hef	Fire attack from more than 1 side: ≥ 300	accordance t	M10x25	0,50	0,43	0,33	0,27	1.53	1,32	1,02	0,81	1,00	0 56	00'0	0.45	2	ure may be de		
fire exposure in	M8×40	0.27	0.25	0.19	0.14		0	00'1	1,20		1,82	1,46		120		Ĩ	Fire att	exposure in a		0,27	0,25	0,19	4	0.67	0,60	0,47	0,34	1,00	1 0.7	70'1	1.46		der fire exposi	≤ R120)	
ad under fire	M8×30	0.27	0.25	0.19	0.14		00 0	00'0	0,70		0,89	0,71		06				load under fire	<b>M8x30</b>	0,27	0,25	0,19	0, 14	0.67	0,60	0,47	0,34	1,00	00 0	0,03	0.71		ce in concrete C20/25 to C50/60 under fire exposure may be determined by: $V^0_{Rk,c,fi}$	0,20 × V <sup>0</sup> <sub>Rk,c</sub> (≤ R120)	
values for tension load under	M6x25	021	0.19	0.15	0.11		1 26	C7.	1,00		0,56	0,45		100				for shear	M6x25	0,21	0,19	0,15 14	- 6	0.40	0,36	0,28	0,20	1,00	0 56	00.0	0.45		ncrete C20/25	-	
				[KN]	1			[kN]	-		[k N]	_1	,	[ m m		[mm]		eristic values			[kN]							Ξ		[kN]	1		sistance in co		
Characteristic			-	NRk,s,fi				N <sub>Rk,p,fi</sub>			N° <sub>Rk,c,fi</sub>		S <sub>cr.f</sub>	Smin	Corf	E'D'	C <sub>min</sub>	Characteristic			V <sub>Rk.s.fi</sub>				074	M Rk,s,fi		k=k <sub>3</sub>		V° <sub>Rk,cp,fi</sub>			haracteristic re		
		R30	R60	R90	R120		R30	R90	R120	failure	R60 R90	R120		1					out lever arm	R30	R60	R90 D120	A IZU	R30	R60	R90	K120		R30	R90	R120	nilure	V <sup>0</sup> <sub>Rk,c,fi</sub> of the cl		
	Steel failure		Characteristic	resistance		Pullout failure	Characteristic	concrete ≥		-	Cnaracteristic resistance in concrete >	C20/25		Spacing		Edge distance			Steel failure without lever arm		Characteristic	resistance	Steel failure with		Characteristic	resistance	Prvout failure	k-factor	Characteristic	concrete ≥	C20/25	Concrete edge failure	The initial value $V^0_{Rk,c, fi}$ of the characteristic resistan		
								M	EA	A D	ROI	<b>P-I</b>	N .	AN	NC	CH(	JF	2											-	Те		of	nn Eur	rop	ean

## Table C4: Reaction to fire

HARMONIZED TECHNICAL SPECIFICATION: ETAG 001 PART 1 PARAGRAPH 5.2.1	
ESSENTIAL CHARACTERISTICS	PERFORMANCE
Reaction to fire	In the final application, the thickness of the mortar layer is about 1 to 2 mm and most of the mortar is material classified class A1 according to EC Decision 96/603/EC. Therefore, it may be assumed that the bonding material (synthetic mortar or a mixture of synthetic mortar and cementitious mortar) in connection with the metal anchor in the end use application do not contribute to fire growth or to the fully developed fire and they have no influence to the smoke hazard.

## MEA DROP-IN ANCHOR

Performance for exposure to fire

Annex C5 of European Technical Assessment ETA-18/0269