

# **Declaration of Performance**

## 1404-CPR-3110

1. Unique identification code of the product-type: Torque-controlled expansion anchor Mungo MSL made of galvanised steel for use in concrete

2. Manufacturer: Mungo Befestigungstechnik AG, Bornfeldstrasse 2, CH-4600 Olten/Switzerland

3. System/s of AVCP: System 1

4. Intended use or use/es:

Product	Intended use
Metal anchor for use in concrete	For fixing and/or supporting to concrete, structural elements (which
	contributes to the stability of the works) or heavy units

5. European Assessment Document: EAD 330232-00-0601, edition October 2016

European Technical Assessment: ETA-18/0653 issued on 25.09.2018

**Technical Assessment Body: ZAG** Notified body/ies: NB 1404

6. Declared performance:

Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic tension resistance acc. EOTA TR 055	See appendix Annex C1
Characteristic shear resistance acc. EOTA TR 055	See appendix Annex C2
Characteristic tension resistance acc. CEN/TS 1992-4	See appendix Annex C1
Characteristic shear resistance acc. CEN/TS 1992-4	See appendix Annex C2
Characteristic resistance under seismic action cat. C1 acc. TR045	See appendix Annex C3
Characteristic resistance under seismic action cat. C2 acc. TR045	See appendix Annex C4

Safety in case of fire (BWR 2)

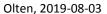
Essential characteristic	Performance
Characteristic tension resistance under fire acc. TR020	See appendix Annex C5
Characteristic shear resistance under fire acc. TR020	See appendix Annex C5

The performance of the product identified above is in conformity with the set of declared performance/s. This declaration of performance is issued, in accordance with Regulation (EU) No 305/2011, under the sole responsibility of the manufacturer identified above.

Singed for and on behalf of the manufacturer by:

Dipl.-Ing. Massimo Pirozzi Head of Engineering

p.p.a. Naimo Dirapi





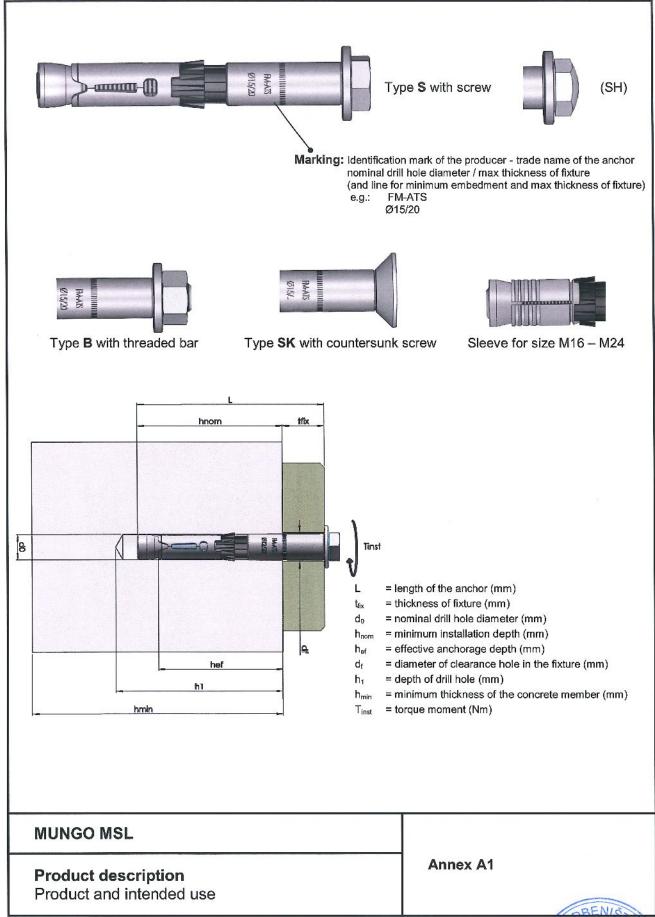
mungo@mungo.swiss

This DoP Has been prepared in different languages. In case there is a dispute on the interpretation the English version shall always prevail.

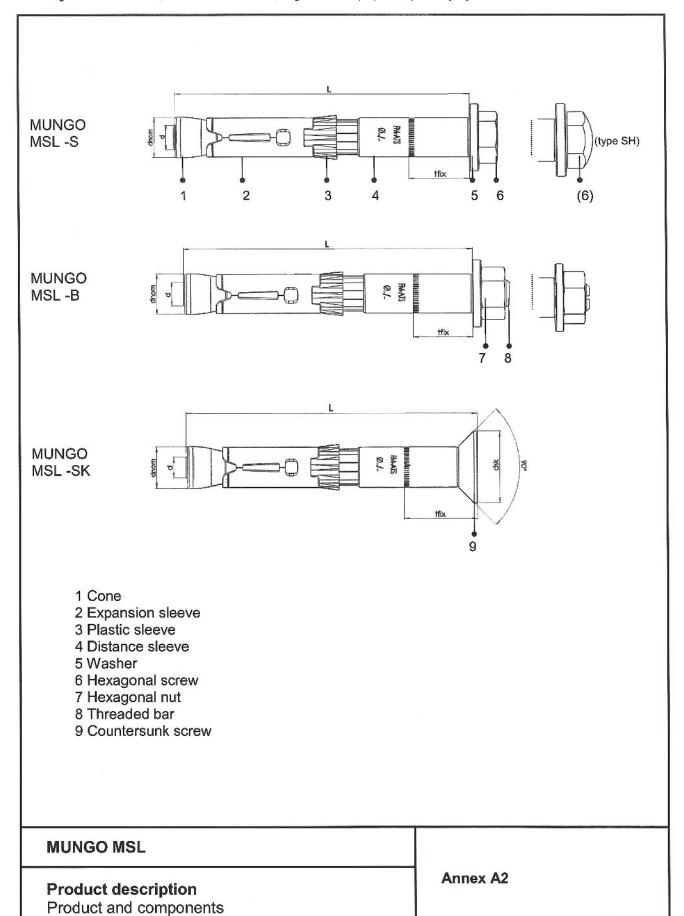
The Appendix includes voluntary and complementary information in English language exceeding the (language as neutrally specified) legal requirements.

Bornfeldstrasse 2 Phone +41 62 206 75 75 Mungo Befestigungstechnik AG

CH-4600 Olten · Switzerland +41 62 206 75 85 Fax



AZ LJUBLJANA ON 17 PARTY - JUBLIANA



DE LJUBLJANA VS. 17 ACT.

Table A1: Materials

Part	of anchor	Material
1	Cone	hardened steel EN 10087 (EN 10277) 1)
2	Expansion sleeve	M6 - M12 hardened steel acc. to EN 10132 <sup>1)</sup> M16 - M24 steel acc. to EN 10087 (EN 10277) <sup>1)</sup>
3	Plastic sleeve	Pa6 acc. to ISO 1874/1
4	Distance sleeve	Steel acc. to EN 10025 1)
5	Washer	Steel acc. to EN 10139 1)
6	Hexagon screw	Steel grade 8.8 acc. to EN ISO 898/11) (DIN 931 -DIN 933 - type SH= large head) 1)
7	Hexagonal nut	Steel grade 8 acc. to EN ISO 898/2 (DIN 934) 1)
8	Threaded bar	Steel grade acc. to 8.8 EN ISO 898/1 1)
9	Countersunk screw	Steel grade acc. to 8.8 EN ISO 898/1 1)

<sup>1)</sup> Zinc plated 5µm according to EN ISO 4042

MUNGO MSL	
Product description Materials	Annex A3

## Specifications of intended use

## Anchorages subjected to:

Static, quasi static, seismic load and fire.

#### Base materials:

- Cracked and non-cracked concrete.
- Reinforced and unreinforced normal weight concrete of strength class C20/25 at minimum and C50/60 at maximum according to EN 206.

# Use conditions (Environmental conditions):

Structures subjected to dry internal conditions.

### Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static and quasi-static actions are designed in accordance with EOTA TR 055, Edition December 2016 or CEN/TS 1992-4.
- For seismic application the anchorages are designed in accordance with TR 045 "Design of metal anchors for use in concrete under seismic actions".
- For application with resistance under fire exposure the anchorages are designed in accordance with method given in TR 020 "Evaluation of anchorage in concrete concerning resistance to fire".
- Verifiable calculation notes and drawings are prepared taking into account of the load to be anchored. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.).

#### Installation:

- Anchor installation carried out by appropriately qualified personnel and under supervision of the person responsible for technical matters of the site.
- Use of the anchor only supplied by the manufacturer without exchanging the components of an anchor.
- Anchor installation in accordance with the manufacturer's specification and drawings and using the appropriate tools.
- Checks before placing the anchor to ensure that the strength class of the concrete in which
  the anchor is to be placed is in the rang given and is not lower that of the concrete to which
  the characteristic loads apply for.
- Check of concrete being well compacted, e.g. without significant voids.
- Effective anchorage depth, edge distances and spacing not less than the specified values without minus tolerances.
- Hole drilling by hammer drill.
- Cleaning of the hole of drilling dust.
- Positioning of the drill holes without damaging the reinforcement.
- Application of specified torque moment using a calibrated torque wrench.
- In case of aborted hole, drilling of new hole at a minimum distance of twice the depth of the aborted hole, or smaller distance provided the aborted drill hole is filled with high strength mortar and no shear or oblique tension loads in the direction of aborted hole.

MUNGO MSL	
Intended use Specification	Annex B1

S LJUBLJANA VO 17 TANA

Table B1: Dimensions

Anchor size			M6	M8	M10	M12	M16	M20	M24
Nominal diameter of anchor d <sub>nom</sub> [mm]		10	12	15	18	24	28	32	
Minimum installation	ı depth	h <sub>nom</sub> ≥ [mm]	60	70	80	100	115	145	165
Length of the anchor L[mm]		t <sub>fix</sub> + 60	t <sub>fix</sub> + 70	t <sub>fix</sub> + 80	t <sub>fix</sub> + 100	t <sub>fix</sub> + 115	t <sub>fix</sub> + 145	t <sub>fix</sub> + 165	
	Type S (SH) /B	t <sub>fix,min</sub> [mm]	0	0	0	0	0	0	0
Thickness of the fixture	Type SK	t <sub>fix,min</sub> [mm]	5	6	6	8	-	-	
	Type S (SH)/B/	SK t <sub>fix,max</sub> [mm]	200	250	300	350	400	450	500
Nominal diameter of the head of the countersunk screw		d <sub>sk</sub> [mm]	17	21	26	31	· ·	-	-

**Intended use**Dimensions of the anchors

Annex B2



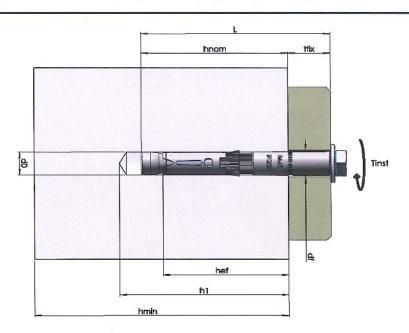


Table B2: Installation data

Anchor size		M6	M8	M10	M12	M16	M20	M24
Nominal drill hole diameter	d₀ [mm]	10	12	15	18	24	28	32
Cutting diameter of drill bit	d <sub>cut</sub> ≤ [mm]	10,45	12,50	15,50	18,50	24,55	28,55	32,55
Depth of drill hole	h₁≥ [mm]	75	85	95	115	130	160	180
Minimum installation depth	h <sub>nom</sub> ≥ [mm]	60	70	80	100	115	145	165
Effective anchorage depth	h <sub>ef</sub> [mm]	49	59	67	88	99	125	150
Diameter of clearance hole in the fixture	d <sub>f</sub> ≤ [mm]	12	14	17	20	26	31	35
Length of the anchor	L [mm]	t <sub>fix</sub> + 60	t <sub>fix</sub> + 70	t <sub>fix</sub> + 80	t <sub>fix</sub> + 100	t <sub>fix</sub> + 115	t <sub>fix</sub> + 145	t <sub>fix</sub> + 165
Torque moment	T <sub>inst</sub> [Nm]	10	20	45	80	150	170	200

Table B3: Minimum thickness of concrete member spacing, and edge distances

Anchor size		M6	M8	M10	M12	M16	M20	M24
Minimum thickness of the concrete member	h <sub>min</sub> [mm]	100	120	140	180	200	250	300
Minimum spacing	S <sub>min</sub> [mm]	50	60	70	80	100	125	150
	for c [mm] ≥	75	90	100	150	200	250	300
Minimum edge distance -	c <sub>min</sub> [mm]	50	60	70	80	100	125	150
	for $s \ge [mm]$	75	90	100	150	200	250	300

8.61	III.I	01	~ E		
Mι	JN	G	JI	VI.	oL.

Intended use Installation parameters Annex B3



Table C1: Characteristic values for Tension loads in case of static and quasi-static loading for design according to EOTA TR 055 or CEN/TS1992-4

Essential cha	ractoristics					Perform				
	DELETED S 69 (2004 A 4 9 C 193 C		M6	M8	M10	M12	M16	M20	M24	
Installation p										
d <sub>o</sub>	Nominal diameter of drill bit	[mm]	10	12	15	18	24	28	32	
h <sub>nom</sub>	Anchorage depth	[mm]	60	70	80	100	115	145	165	
h <sub>ef</sub>	Effective anchorage depth	[mm]	49	59	67	88	99	125	150	
h <sub>min</sub>	Minimum thickness of concrete member	[mm]	100	120	140	180	200	250	300	
Tinst	Torque moment	[Nm]	10	20	45	80	150	170	200	
Smin	Minimum spacing	[mm]	50	60	70	80	100	125	150	
for c ≥	Edge distance	[mm]	75	90	100	150	200	250	300	
Cmin	Minimum edge distance	[mm]	50	60	70	80	100	125	150	
for s ≥	Spacing	[mm]	75	90	100	150	200	250	300	
Tension stee	I failure mode	1 2 5 5								
N <sub>Rk,s</sub>	Characteristic tension steel failure	[kN]	16	29	46	67	126	203	293	
ΎMsN	Partial safety factor	[-]		\$\$	* 3	1,5			t.	
Pull-out failu		A 12 3 - 3 -								
N <sub>Rk,p</sub>	Characteristic pull-out failure in non- cracked concrete	[kN]	_1)	_1)	<b>-</b> 1)	_1)	_1)	_1)	_1)	
N <sub>Rk,p</sub>	Characteristic pull-out failure in cracked concrete	[kN]	9	12	16	25	_1)	_1)	_1)	
γ2		[-]	1,0							
ум <sub>р</sub>	Partial safety factor	[-]				1,5				
Scr.N	Characteristic spacing	[mm]				3 x h	of			
C <sub>cr.N</sub>	Characteristic edge distance	[mm]				1,5 x				
ψc C30/37	Onaradionolio dago distanto	[-]	1,22							
ψc C40/50	Increasing factor for N <sub>Rk,p</sub> for concrete	[-]								
	increasing factor for takk,p for concrete		1,41 1,55							
ψc C50/60	ne failure mode	[-]				1,00	)			
Concrete Co	Factor for cracked concrete CEN/TS 1992-									
k <sub>cr</sub>	4-4 §. 6.2.1.4	[-]				7,2				
Kucr	Factor for un-cracked concrete CEN/TS 1992-4-4 §. 6.2.1.4	[-]				10,1				
γMc	Partial safety factor	[-]				1,5				
Splitting failu			-0.57							
Scr,sp	Characteristic spacing	[mm]				3 x h				
C <sub>cr,sp</sub>	Characteristic edge distance	[mm]				1,5 x				
γмѕр	Partial safety factor	[-]	1,5							
Displacemen	t under tension load									
Non-cracked	concrete C20/25									
N	Service tension load	[kN]	7,7	10,9	13,2	19,8	23,6	33,6	44,2	
δηο	Short term displacement	[mm]	0,47	0,81	0,30	0,25	0,20	2,08	2,45	
$\delta_{N_{\infty}}$	Long term displacement	[mm]	2,38	2,49	1,99	1,12	2,15	2,08	2,45	
Cracked cond							100	10 10	-	
N	Service tension load	[kN]	4,3	5,7	7,6	11,9	16,9	23,9	31,5	
δηο	Short term displacement	[mm]	1,21	0,83	1,25	0,98	0,96	0,99	1,41	
δ <sub>N∞</sub>	Long term displacement	[mm]	2,38	2,49	1,99	1,12	2,15	0,99	1,41	

1) The pull-out is not decisive

## **MUNGO MSL**

**Design acc. to EOTA TR 055 or CEN/TS 1992-4**Characteristic resistance under Tension loads – BWR 1



Table C2: Characteristic values for Shear loads in case of static and quasi-static loading for design according to EOTA TR 055 or CEN/TS 1992-4

Ftiel above stavistics			Performance							
Essential characteristics			M6	M8	M10	M12	M16	M20	M24	
Shear ste	el failure mode									
V <sub>Rk,s</sub>	Characteristic shear steel failure	[kN]	14	26	42	50	97	125	151	
M <sup>0</sup> Rk,s	Bending moment characteristic failure	[Nm]	12	30	60	105	266	542	932	
γMsV	Partial safety factor	[-]				1,25	,			
	crete pry-out and edge failure									
<b>K</b> <sub>3</sub>	Factor in equation (16) of CEN/TS 1992-4 § 6.2.2.3	[-]	1,0		2,0					
lef	Effective anchorage depth	[mm]	46	59	67	88	99	125	150	
dnom	Diameter of anchor	[mm]	10	12	15	18	24	28	32	
Displacen	nent under tension load					1.				
Non-crack	ed concrete C20/25						= "+1"			
٧	Service shar load	[kN]	8,0	14,9	24,0	28,6	55,4	71,4	86,3	
δνο	Short term displacement	[mm]	1,39	1,94	2,71	1,69	2,69	7,84	8,87	
δν <sub>∞</sub>	Long term displacement	[mm]	2,09	2,91	4,07	2,54	4,04	11,76	13,3	

Design acc. to EOTA TR 055 or CEN/TS 1992-4 Characteristic resistance under Shear loads – BWR 1



Table C3: Characteristic values for resistance in case of Seismic performance category C1 acc. TR045 "Design of Metal anchor under Seismic Actions"

Essential characteristics			Performance								
Essential characteristics			M6	M8	M10	M12	M16	M20	M24		
Tension ste	el failure										
NRk,s,seis C1	Characteristic tension steel failure	[kN]	16	29	46	67	126	203	293		
YMsN,seis <sup>2)</sup>	Partial safety factor	[-]	1,5								
Pull-out fail	ure mode N <sub>Rk,p,seis</sub> = ψ <sub>C</sub> × N <sup>0</sup> <sub>Rk,p,seis</sub>								//_/I		
NRk,p,seis C1	Characteristic pull-out failure in concrete C20/25	[kN]	6,8	12	16	25	35,51)	50,21	66,11		
γMp,seis <sup>2)</sup>	Partial safety factor	[-]	1,5								
Shear steel	failure			BUSIN		T.					
V <sub>Rk,s,seisC1</sub>	Characteristic shear steel failure	[kN]	9,8	13	20	20	48,5	87,5	105,7		
γMsV,seis <sup>2)</sup>	Partial safety factor	[-]	1,25								

<sup>1)</sup> The pull-out is not decisive

Design according to TR 045

Characteristic resistance under Seismic actions - BWR 1



<sup>&</sup>lt;sup>2)</sup> The recommended partial safety factors under seismic action ( $\gamma_{M,seis}$ ) are the same as for static loading

Table C4: Characteristic values for resistance in case of Seismic performance category C2 acc. TR045 "Design of Metal anchor under Seismic Actions"

Essential characteristics			Performance								
			M6	M8	M10	M12	M16	M20	M24		
Tension stee	el failure										
NRk,s,seis C2 <sup>2)</sup>	Characteristic tension steel failure	[kN]	16	29	46	67	126	203	293		
YMsN <sup>3)</sup>	Partial safety factor	[-]	1,5								
Pull-out failu	ire $N_{Rk,p,seis} = \psi_C \times N_{Rk,seis}$										
NRk,p,seis C2 <sup>2)</sup>	Characteristic pull-out failure in concrete C20/25	[kN]	=	3,9	7,8	15,3	28,8	32,8	41,3		
<b>У</b> МрN <sup>3)</sup>	Partial safety factor	[-]	1,5								
δ <sub>N,sel(DSL)</sub> 1)2)	Displacement at DSL	[mm]	-	2,7	4,9	3,6	3,1	7,0	7,0		
δ <sub>N,sei(USL)</sub> 1)2)	Displacement at USL	[mm]	-	12,8	15,2	14,0	11,5	18,4	16,2		
Shear steel f	failure						6/X				
V <sub>Rk,s,seis</sub> C2 <sup>2)</sup>	Characteristic shear failure	[kN]		10,2	17,0	17,0	43,9	72,9	74,6		
γMsv <sup>3)</sup>	Partial safety factor	[+]	1,25								
δ <sub>V,sei(DSL)</sub> 1)2)	Displacement at DSL	[mm]		3,5	2,7	2,5	2,7	7,0	7,0		
δv,sei(USL) <sup>1)2)</sup>	Displacement at USL	[mm]	-	6,8	6,3	5,8	6,1	20,9	18,6		

<sup>1)</sup> The listed displacement represent mean values

Design according to TR 045

Characteristic resistance under Seismic actions - BWR 1



<sup>&</sup>lt;sup>2)</sup> A smaller displacement may be required in the design in the case of displacement sensitive fastenings or "rigid" supports. The characteristic resistance associated with such smaller displacement may be determined by linear interpolation or proportional reduction.

<sup>3)</sup> The recommended partial safety factors under seismic action (y<sub>M,sels</sub>) are the same as for static loading

Table C5: Characteristic resistance under Fire exposure for design acc. to TR020

Essential characteristics			Performance							
		M6	M8	M10	M12	M16	M20	M24		
Tension stee	el failure mode									
N <sub>Rk,s,fi,30</sub>	Duration = 30 minutes	[kN]	0,20	0,37	0,87	1,69	3,14	4,90	7,06	
NRk,s,fi,60	Duration = 60 minutes	[kN]	0,18	0,33	0,75	1,26	2,36	3,68	5,30	
N <sub>Rk,s,fi,90</sub>	Duration = 90 minutes	[kN]	0,14	0,26	0,58	1,10	2,04	3,19	4,59	
NRk,s,fi,120	Duration = 120 minutes	[kN]	0,10	0,18	0,46	0,84	1,57	2,45	3,53	
Pull-out failu							lavo.			
N <sub>Rk,p,fi,30</sub>	Duration = 30 minutes	[kN]	2,25	3,00	4,00	6,25	8,88	12,58	16,54	
NRk,p,fi,60	Duration = 60 minutes	[kN]	2,25	3,00	4,00	6,25	8,88	12,58	16,54	
NRk,p,fi,90	Duration = 90 minutes	[kN]	2,25	3,00	4,00	6,25	8,88	12,58	16,54	
NRk,p,fi,120	Duration = 120 minutes	[kN]	1,80	2,40	3,20	5,00	7,10	10,06	13,23	
	ne failure mode									
N <sub>Rk,c,fi,30</sub>	Duration = 30 minutes	[kN]	3,03	4,81	6,61	13,08	17,55	31,44	49,6	
N <sub>Rk,c,fi,60</sub>	Duration = 60 minutes	[kN]	3,03	4,81	6,61	13,08	17,55	31,44	49,6	
N <sub>Rk,c,fi,90</sub>	Duration = 90 minutes	[kN]	3,03	4,81	6,61	13,08	17,55	31,44	49,6	
NRk,c,fi,120	Duration = 120 minutes	[kN]	2,42	3,85	5,29	10,46	14,04	25,16	39,68	
Scr,N	Characteristic spacing	[mm]	4 x h <sub>ef</sub>							
C <sub>cr,N</sub>	Characteristic edge distance	[mm]	2 x h <sub>ef</sub>							
Smin	Minimum spacing	[mm]	50	60	70	80	100	125	150	
Cmin	Minimum edge distance	[mm]	$c_{min} = 2 \; h_{ef};$ if fire attack from more than one side, the edge distance of the anchor has to be $\geq 300$ mm and $\geq 2 \; h_{ef}$							
Ум, <del>f</del> i	Partial safety factor	[-]	1,01)							
	failure without lever arm									
V <sub>Rk,s,fi,30</sub>	Duration = 30 minutes	[kN]	0,20	0,37	0,87	1,69	3,14	4,9	7,06	
VRk,s,fi,60	Duration = 60 minutes	[kN]	0,18	0,33	0,75	1,26	2,36	3,68	5,30	
V <sub>Rk,s,fi,90</sub>	Duration = 90 minutes	[kN]	0,14	0,26	0,58	1,10	2,04	3,19	4,59	
V <sub>Rk,s,fi,120</sub>	Duration = 120 minutes	[kN]	0,10	0,18	0,46	0,84	1,57	2,45	3,53	
	failure with lever arm		7 E			# #4			are said	
M <sup>0</sup> Rk,s,fi,30	Duration = 30 minutes	[Nm]	0,15	0,37	1,12	2,62	6,66	13,07	22,4	
M <sup>0</sup> Rk,s,fi,60	Duration = 60 minutes	[Nm]	0,14	0,34	0,97	1,96	5,00	9,80	16,8	
M <sup>0</sup> Rk,s,fi,90	Duration = 90 minutes	[Nm]	0,11	0,26	0,75	1,70	4,33	8,49	14,5	
M <sup>0</sup> Rk,s,fi,120	Duration = 120 minutes	[Nm]	0,08	0,19	0,60	1,31	3,33	5,44	9,35	
-55 OC 55 OC	rete pry-out failure									
К3	Factor in equation (16) of CEN/TS 1992-4 § 6.2.2.3	[mm]	1,0 2,0							

The characteristic resistance  $V^0_{Rk,c,fi}$  in C 20/25 to C5 0/60 concrete is determined by:

 $V_{Rk,c,fi} = 0.25 \times V_{Rk,c} (\le R90)$  and  $V_{Rk,c,fi} = 0.20 \times V_{Rk,c} (R120)$ 

with Vo<sub>Rko</sub> initial value of the characteristic resistance in cracked concrete C20/25 under normal temperature.

# **MUNGO MSL**

Design according to TR020

Characteristic resistance under Fire exposure - BWR 2



<sup>1)</sup> In absence of other national regulations