

## **Declaration of Performance**

### 0679-CPR-0832

**1. Unique identification code of the product-type:** Torque-controlled expansion anchor m1t- for use in cracked and uncracked concrete

2. Manufacturer: Mungo Befestigungstechnik AG, Bornfeldstrasse 2, CH-4603 Olten/Switzerland

3. System/s of AVCP: System 1

### 4. Intended use or use/es:

Product	Intended use
Metal anchor for use in concrete	For fixing and/or supporting to concrete, structural elements (which
	contributes to the stability of the works) or heavy units

5. European Assessment Document: ETAG 001, Edition April 2013 used as EAD

European Technical Assessment: ETA-12/0547 of 29/04/2014

Technical Assessment Body: CSTB-Centre Scientifique et Technique du Batiment

Notified body/ies: n°0679 – CSTB le futur en construction

### 6. Declared performance:

### Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic tension resistance acc. ETAG001, Annex C	See appendix, especially Annex C1
Characteristic shear resistance acc. ETAG001, Annex C	See appendix, especially Annex C2
Characteristic tension resistance acc. CEN/TS 1992-4	See appendix, especially Annex C5
Characteristic shear resistance acc. CEN/TS 1992-4	See appendix, especially Annex C6
Characteristic resistance under seismic action Cat 1 acc. TR045	See appendix, especially Annex C9
Characteristic resistance under seismic action Cat 2 acc. TR045	See appendix, especially Annex C10
Displacement	See appendix, especially Annex C11

### Safety in case of fire (BWR 2)

suicty in case of the (Bittit =)	
Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class
	A1
Characteristic tension resistance under fire acc. ETAG001, Annex C	See appendix, especially Annex C3
Characteristic shear resistance under fire acc. ETAG001, Annex C	See appendix, especially Annex C4
Characteristic tension resistance under fire acc. CEN/TS 1992-4	See appendix, especially Annex C7
Characteristic shear resistance under fire acc. CEN/TS 1992-4	See appendix, especially Annex C8

The performance of the product identified above is in conformity with the set of declared performance/s. This declaration of performance is issued, in accordance with Regulation (EU) No 305/2011, under the sole responsibility of the manufacturer identified above.

Singed for and on behalf of the manufacturer by:

Dipl.-Ing. Massimo Pirozzi Head of Engineering

p.p.a. Maino Dirapi

Olten, 2017-22-12



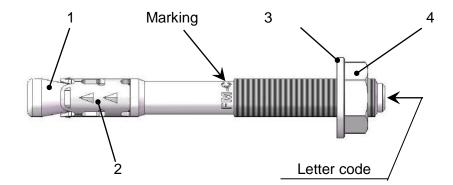
This DoP Has been prepared in different languages. In case there is a dispute on the interpretation the English version shall always prevail.

The Appendix includes voluntary and complementary information in English language exceeding the (language as neutrally specified) legal requirements.

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### Assembled anchor:



- 1. Bolt
- 2. Expansion sleeve
- 3. Washer
- 4. Hexagonal nut

Marking on the bolt:

FM-C (product name) followed by MX/Y where

MX = thread diameter Y = fixture thickness

**Table 1: Materials** 

Part	Designation	Material	Protection		
1	Bolt	M8 and M10: 19MnB4 DIN 1654-T4			
'	Boit	M12 and M16 C30BKD EU 119-74	Galvanised <sup>1)</sup> ≥ 8μm		
2	Expansion sleeve	Stainless steel X2CrNiMo 17-12-2 UNI EN 10088/2	-		
3	Washer	C-steel DIN 125/1 (normal), DIN 9021 (large)	Galvanised <sup>1)</sup> ≥ 8µm		
4	Hexagonal nut	C-steel DIN 934, steel grade 8	Galvanised <sup>1)</sup> ≥ 8µm		

<sup>1)</sup> Special galvanised NAUTILUS Brilliant

Mungo m1t- expansion anchor	
Product description	Annex A1
Installation condition - Materials	

### Specifications of intended use

### Anchorages subject to:

- Static and quasi-static loads,
- Seismic load (category C2) loads,
- Fire.

### Base materials:

- Cracked concrete and non-cracked concrete
- Reinforced or unreinforced normal weight concrete of strength classes C 20/25 at least to C50/60 at most according to ENV 206: 2000-12.

### **Use conditions (Environmental conditions):**

Structures subject to dry internal conditions.

### Design:

- The anchorages are designed in accordance with the ETAG001 Annex C "Design Method for Anchorages" or CEN/TS 1992-4-4 " Design of fastenings for use in concrete" under the responsibility of an engineer experienced in anchorages and concrete work.
- For seismic application the anchorages are designed in accordance with TR045 "Design of Metal Anchors For Use In Concrete Under Seismic Actions".
- For application with resistance under fire exposure the anchorages are designed in accordance with method given in TR020 "Evaluation of Anchorage in Concrete concerning Resistance to Fire".
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings.

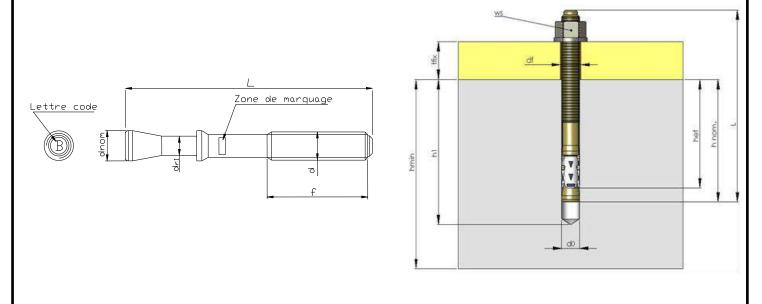
### Installation:

- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- Use of the anchor only as supplied by the manufacturer without exchanging the components of an anchor.
- Anchor installation in accordance with the manufacturer's specifications and drawings and using the appropriate tools.
- Effective anchorage depth, edge distances and spacing not less than the specified values without minus tolerances.
- Hole drilling by hammer drill.
- Cleaning of the hole of drilling dust.
- Application of specified torque moment using a calibrated torque wrench.
- In case of aborted hole, drilling of new hole at a minimum distance of twice the depth of the aborted hole, or smaller distance provided the aborted drill hole is filled with high strength mortar and no shear or oblique tension loads in the direction of aborted hole.

Mungo m1t- expansion anchor	
Intended Use Specifications	Annex B2

**Table 2: Anchor dimensions** 

	d x L	Marking	Letter code ID	<b>L</b>	d <sub>nom</sub>	d <sub>r1</sub>	f
	M8x68	FM-C 8/4	A	(mm) 68	(mm)	(mm)	(mm) 30
	M8x75	FM-C 8/10	В	75			30
<b>~</b>	M8x90	FM-C 8/25	С	90			40
<b>M</b>	M8x115	FM-C 8/50	D	115	8	5,8	60
	M8x135	FM-C 8/70	Е	135			80
	M8x165	FM-C 8/100	G	165			80
	M10x90	FM-C 10/10	Α	90			40
	M10x105	FM-C 10/25	В	105	10	7,4	55
M10	M10x115	FM-C 10/35	С	115			55
Σ	M10x135	FM-C 10/55	D	135			85
	M10x155	FM-C 10/75	Е	155			85
	M10x185	FM-C 10/105	F	185			85
	M12x110	FM-C 12/10	Α	110			65
~	M12x120	FM-C 12/20	В	120			65
M12	M12x145	FM-C 12/45	С	145	12	8,8	85
_	M12x170	FM-C 12/70	D	170			85
	M12x200	FM-C 12/100	Е	200			85
	M16x130	FM-C 16/10	Α	130	16		65
M16	M16x150	FM-C 16/30	В	150		11,8	85
Σ	M16x185	FM-C 16/60	С	185		11,0	85
	M16x220	FM-C 16/100	D	220			85



# Mungo m1t- expansion anchor Intended Use Installation parameters Annex B3

Table 3: Installation data

	dxL	ID	t <sub>fix</sub>	<b>d</b> <sub>0</sub> (mm)	<b>h</b> <sub>1</sub> (mm)	h <sub>nom</sub>	h <sub>ef</sub>	d <sub>f</sub>	h <sub>min</sub>	T <sub>inst</sub> (Nm)	WS (mm)	Marking						
	M8x68	Α	4									FM-C 8/4						
	M8x75	В	10									FM-C 8/10						
8 <u>W</u>	M8x90	С	25	8	70	54	48	9	100	20	13	FM-C 8/25						
2	M8x115	D	50	O	0 70 34 40 3 100 20	13	FM-C 8/50											
	M8x135	Е	70								FM-C 8/70							
	M8x165	G	100									FM-C 8/100						
	M10x90	Α	10									FM-C 10/10						
	M10x105	В	25	10 80	80	80 67	67 60	60 12	120	0 40	17	FM-C 10/25						
M10	M10x115	С	35									FM-C 10/35						
Σ	M10x135	D	55									FM-C 10/55						
	M10x155	Е	75															FM-C 10/75
	M10x185	F	105									FM-C 10/105						
	M12x110	Α	10									FM-C 12/10						
<b>Q</b>	M12x120	В	20												FM-C 12/20			
M12	M12x145	С	45	12	100	81	72	14	150	60	19	FM-C 12/45						
	M12x170	D	70									FM-C 12/70						
	M12x200	Е	100									FM-C 12/100						
	M16x130	Α	10									FM-C 16/10						
M16	M16x150	В	30	16	115	97	86	18	170	120	24	FM-C 16/30						
Σ	M16x185	С	60	10	113	91	00	18	170	120	<b>4</b>	FM-C 16/60						
	M16x220	D	100									FM-C 16/100						

			M8	M10	M12	M16
Min. member thickness	h <sub>min</sub>	[mm]	100	120	150	170
Minimum edge distance	C <sub>min</sub>	[mm]	50	60	70	85
Corresponding spacing	s≥	[mm]	75	120	150	170
Minimum spacing	S <sub>min</sub>	[mm]	50	60	70	80
Corresponding edge distance	c≥	[mm]	65	80	90	120

Mungo m1t- expansion anchor	
Intended Use Installation parameters	Annex B3

Table 4: Characteristic values for tension loads in case of static and quasi static loading for design method A acc.  $\overline{\text{ETAG001}}$ , Annex C

			M8	M10	M12	M16
Steel failure						
Char. resistance	$N_{Rk,s}$	[kN]	23,8	38,7	54,7	98,4
Partial safety factor	γ <sub>Ms</sub> <sup>1)</sup>	[-]	1,5			

Pullout failure $N_{Rk,p} = \Psi_c \times N^0_{Rk,p}$									
Char. resistance in	non-cracked	$N^0_{Rk,p}$	[kN]	9	16	20	35		
concrete C20/25	cracked	$N^0_{Rk,p}$	[kN]	6	12	16	20		
Partial safety factor for cracked or non-cracked concrete		γ <sub>Mp</sub> <sup>1)</sup>	[-]	1,5 <sup>2)</sup>					
concrete C30/37			[-]		1	,22			
Increasing factor for N <sub>RK</sub>	concrete C40/50	$\Psi_{\mathtt{c}}$	[-]		1	,41			
141	concrete C50/60		[-]		1,55				

Concrete cone failure and splitting failure								
Effective embedment depth		h <sub>ef</sub>	[mm]	48	60	72	86	
Partial safety factor for craked or non-crace	cked concrete	$\gamma_{Mc} = \gamma_{Msp}^{1)}$		1,5 <sup>2)</sup>				
	concrete C30/37		[-]		1,22			
Increasing factor for N <sub>RK</sub>	concrete C40/50	$\Psi_{c}$	[-]		1,41			
· · · · · · · · · · · · · · · · · · ·	concrete C50/60		[-]	1,55				
Char. spacing	concrete cone failure	S <sub>cr,N</sub>	[mm]	140	180	220	260	
Onar. opaoing	splitting failure	S <sub>cr,sp</sub>	[mm]	290	360	430	520	
Char. edge distance	concrete cone failure	C <sub>cr,N</sub>	[mm]	70	90	110	130	
Onar. edge distance	splitting failure	C <sub>cr,sp</sub>	[mm]	145	180	215	260	

# Mungo m1t- expansion anchor Annex C1 Design according to ETAG001, Annex C Characteristic resistance under tension loads

 $<sup>^{1)}</sup>$  In absence of other national regulations  $^{2)}$  The value contains an installation safety factor  $\gamma_{2}\text{=}\ 1.0$ 

Table 5: Characteristic values for shear loads in case of static and quasi static loading for design design method A acc.  $\boxed{\text{ETAG001, Annex C}}$ 

			M8	M10	M12	M16
Steel failure without lever arm						
Char. resistance	$V_{Rk,s}$	[kN]	12,9	24,2	33,8	66,4
Partial safety factor	γ <sub>Ms</sub> <sup>1)</sup>	[-]	1,5			

Steel failure with lever arm							
Char. bending resistance	$M^0_{Rk,s}$	[Nm]	34	67	118	300	
Partial safety factor	γ <sub>Ms</sub> 1)	[-]	1,5				

Concrete pry-out failure						
Factor in equation (5.6) of ETAG Annex C, § 5.2.3.3	k	[-]	1,0	2,0	2,0	2,0
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]	1,5 <sup>1)</sup>			

Concrete edge failure							
Effective length of anchor under shear loading	I <sub>f</sub>	[mm]	48	60	72	86	
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16	
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]	1,5 <sup>1)</sup>				

<sup>&</sup>lt;sup>1)</sup> The installation safety factor  $\gamma_2$  =1.0 is included

Mungo m1t- expansion anchor

Design according to ETAG001, Annex C

Characteristic resistance under shear loads

**Annex C2** 

Table 6: Characteristic tension resistance in cracked and non-cracked concrete under fire exposure for design method A acc. ETAG001, Annex C

			M8	M10	M12	M16
Steel failure	· · · · ·					
	R30 N <sub>Rk,s,fi</sub>	[kN]	0,4	0,9	1,7	3,1
Characteristic registance	R60 N <sub>Rk,s,fi</sub>	[kN]	0,3	0,8	1,3	2,4
Characteristic resistance	R90 N <sub>Rk,s,fi</sub>	[kN]	0,3	0,6	1,1	2,0
	R120 N <sub>Rk,s,fi</sub>	[kN]	0,2	0,5	0,8	1,6

Pullout failure (cracked and non-cracked concrete)						
Char. resistance in concrete ≥ C20/25	R30 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0
	R60 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0
	R90 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0
	R120 N <sub>Rk,p,fi</sub>	[kN]	1,2	2,4	3,2	4,0

Concrete cone and splitting failure <sup>2)</sup> (cracked and non-cracked concrete)							
	R30 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3	
Char. resistance in concrete ≥ C20/25	R60 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3	
	R90 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3	
	R120 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,3	4,0	6,3	9,9	
Characteristic spacing	S <sub>cr,N,fi</sub>	[mm]	4 x h <sub>ef</sub>				
Characteristic edge distance	C <sub>cr,N,fi</sub>	[mm]	2 x h <sub>ef</sub>				

<sup>1)</sup> Design under fire exposure is performed according to the design method given in TR 020. Under fire exposure usually cracked concrete is assumed. The design equations are given in TR 020, Section 2.2.1.

TR 020 covers design for fire exposure from one side. For fire attack from more than one side the edge distance must be increased to  $c_{min} \ge 300$  mm and  $\ge 2 \cdot h_{ef}$ .

Mungo m1t- expansion anchor	
Design according to ETAG001, Annex C  Characteristic tension resistance under fire exposure	Annex C3

<sup>&</sup>lt;sup>2)</sup> As a rule, splitting failure can be neglected when cracked concrete and reinforcement is assumed.

Table 7: Characteristic shear resistance in cracked and non-cracked concrete under fire exposure for design method A acc. ETAG001, Annex C

			M8	M10	M12	M16
Steel failure without lever arm			_			
	R30 V <sub>Rk,s,fi</sub>	[kN]	0,4	0,9	1,7	3,1
Characteristic resistance	R60 V <sub>Rk,s,fi</sub>	[kN]	0,3	0,8	1,3	2,4
Characteristic resistance	R90 V <sub>Rk,s,fi</sub>	[kN]	0,3	0,6	1,1	2,0
	R120 V <sub>Rk,s,fi</sub>	[kN]	0,2	0,5	0,8	1,6

Steel failure with lever arm				-		
Characteristic bending moment	R30 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,4	1,1	2,6	6,7
	R60 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,3	1,0	2,0	5,0
	R90 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,3	0,7	1,7	4,3
	R120 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,2	0,6	1,3	3,3

Concrete pry-out failure		•		-	·	
Factor in equation (5.6) of ETAG Annex C, § 5.2.3.3	k	[-]	1,0	2,0	2,0	2,0
	R30 V <sub>Rk,cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
Characteristic resistance	R60 V <sub>Rk, cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
Characteristic resistance	R90 V <sub>Rk, cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
	R120 V <sub>Rk, cp,fi</sub>	[kN]	2,3	8,0	12,7	19,8

Concrete edge failure				-	-	·
Eff. length of anchor under shear loading	l <sub>f</sub>	[mm]	48	60	72	86
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16

Design under fire exposure is performed according to the design method given in TR 020. Under fire exposure usually cracked concrete is assumed. The design equations are given in TR 020, Section 2.2.2.

TR 020 covers design for fire exposure from one side. For fire attack from more than one side the edge distance must be increased to  $c_{min} \ge 300$  mm and  $\ge 2 \cdot h_{ef}$ .

Mungo m1t- expansion anchor	
Design according to ETAG001, Annex C Characteristic shear resistance under fire exposure	Annex C4

Table 8: Characteristic values for tension loads in case of static and quasi static loading for design design method A acc. CEN/TS 1992-4

			M8	M10	M12	M16
Steel failure						
Char. resistance	$N_{Rk,s}$	[kN]	23,8	38,7	54,7	98,4
Partial safety factor	γ <sub>Ms</sub> 1)	[-]	1,5			

Pullout failure $N_{Rk,p} = \Psi_c \times N_{Rk,p}^0$									
Char. resistance in	non-cracked	$N^0_{Rk,p}$	[kN]	9	16	20	35		
concrete C20/25	cracked	$N^0_{Rk,p}$	[kN]	6	12	16	20		
Partial safety factor for cracked or non-cra	cked concrete	γ <sub>Mp</sub> 1)	[-]		1,5 <sup>2)</sup>				
	concrete C30/37		[-]		1,22				
Increasing factor for N <sub>RK,p</sub>	concrete C40/50	Ψς	c [-] 1,41						
. Will	concrete C50/60		[-]		1	,55			

Concrete cone failure and splitting failure									
Effective embedment depth		h <sub>ef</sub>	[mm]	48	60	72	86		
Factor for cracked co	ncrete	k <sub>cr</sub>		7,2					
Factor for non cracke	d concrete	k <sub>ucr</sub>		10,1					
Partial safety factor		$\gamma_{Mc} = \gamma_{Msp}^{1)}$	1,5 <sup>2)</sup>		5 <sup>2)</sup>				
Char. spacing	concrete cone failure	S <sub>cr,N</sub>	[mm]	140	180	220	260		
Onar. spacing	splitting failure	S <sub>cr,sp</sub>	[mm]	290	360	430	520		
Char. edge distance	concrete cone failure	C <sub>cr,N</sub>	[mm]	70	90	110	130		
	splitting failure	C <sub>cr,sp</sub>	[mm]	145	180	215	260		

# Mungo m1t- expansion anchor **Annex C5** Design according to CEN/TS 1992-4 Characteristic resistance under tension loads

 $<sup>^{1)}</sup>$  In absence of other national regulations  $^{2)}$  The value contains an installation safety factor  $\gamma_2 = 1.0$ 

Table 9: Characteristic values for shear loads in case of static and quasi static loading for design design method A acc. CEN/TS 1992-4

			M8	M10	M12	M16		
Steel failure without lever arm								
Char. resistance	$V_{Rk,s}$	[kN]	12,9	24,2	33,8	66,4		
Factor considering ductility	k <sub>2</sub>	[-]	0,8					
Partial safety factor	γ <sub>Ms</sub> <sup>1)</sup>	[-]	1,5					

Steel failure with lever arm	<u>-</u>	<u>-</u>				<u>,                                      </u>	
Char. bending moment	$M^0_{Rk,s}$	[Nm]	34	67	118	300	
Partial safety factor	γ <sub>Ms</sub> <sup>1)</sup>	[-]	1,5				

Concrete pry-out failure						
Factor in equation (16) of CEN TS 1992-4-4, § 6.2.2.3	k <sub>3</sub>	[-]	1,0	2,0	2,0	2,0
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]	1,5 <sup>1)</sup>			

Concrete edge failure								
Effective length of anchor under shear loading	I <sub>f</sub>	[mm]	48	60	72	86		
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16		
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]	1,5 <sup>1)</sup>					

 $<sup>^{1)}</sup>$  The installation safety factor  $\gamma_2$  =1.0 is included

Mungo m1t- expansion anchor

Design according to CEN/TS 1992-4

Characteristic resistance under shear loads

**Annex C6** 

Table 10: Characteristic tension resistance in cracked and non-cracked concrete under fire exposure for design method A acc. CEN/TS 1992-4

			M8	M10	M12	M16
Steel failure						
	R30 N <sub>Rk,s,fi</sub>	[kN]	0,4	0,9	1,7	3,1
Characteristic registance	R60 N <sub>Rk,s,fi</sub>	[kN]	0,3	0,8	1,3	2,4
Characteristic resistance	R90 N <sub>Rk,s,fi</sub>	[kN]	0,3	0,6	1,1	2,0
	R120 N <sub>Rk,s,fi</sub>	[kN]	0,2	0,5	0,8	1,6

Pullout failure (cracked and non-cracked concrete)							
Char. resistance in concrete ≥ C20/25	R30 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0	
	R60 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0	
	R90 N <sub>Rk,p,fi</sub>	[kN]	1,5	3,0	4,0	5,0	
	R120 N <sub>Rk,p,fi</sub>	[kN]	1,2	2,4	3,2	4,0	

Concrete cone and splitting failure <sup>2)</sup> (cracked and non-cracked concrete)								
	R30 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3		
Char. resistance in concrete ≥ C20/25	R60 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3		
	R90 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,9	5,0	7,9	12,3		
	R120 N <sup>0</sup> <sub>Rk,c,fi</sub>	[kN]	2,3	4,0	6,3	9,9		
Characteristic spacing	S <sub>cr,N,fi</sub>	[mm]	4 x h <sub>ef</sub>					
Characteristic edge distance	C <sub>cr,N,fi</sub>	[mm]	2 x h <sub>ef</sub>					

Design under fire exposure is performed according to the design method given in TR 020. Under fire exposure usually cracked concrete is assumed. The design equations are given in TR 020, Section 2.2.1.

TR 020 covers design for fire exposure from one side. For fire attack from more than one side the edge distance must be increased to  $c_{min} \ge 300$  mm and  $\ge 2 \cdot h_{ef}$ .

Mungo m1t- expansion anchor	
Design according to CEN/TS 1992-4 Characteristic tension resistance under fire exposure	Annex C7

<sup>&</sup>lt;sup>2)</sup> As a rule, splitting failure can be neglected when cracked concrete and reinforcement is assumed.

Table 11: Characteristic shear resistance in cracked and non-cracked concrete under fire exposure for design method A acc. CEN/TS 1992-4

			M8	M10	M12	M16
Steel failure without lever arm					-	
Characteristic resistance	R30 V <sub>Rk,s,fi</sub>	[kN]	0,4	0,9	1,7	3,1
	R60 V <sub>Rk,s,fi</sub>	[kN]	0,3	0,8	1,3	2,4
	R90 V <sub>Rk,s,fi</sub>	[kN]	0,3	0,6	1,1	2,0
	R120 V <sub>Rk,s,fi</sub>	[kN]	0,2	0,5	0,8	1,6

Steel failure with lever arm								
Characteristic bending moment	R30 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,4	1,1	2,6	6,7		
	R60 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,3	1,0	2,0	5,0		
	R90 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,3	0,7	1,7	4,3		
	R120 M <sup>0</sup> <sub>Rk,s,fi</sub>	[Nm]	0,2	0,6	1,3	3,3		

Concrete pry-out failure						
Factor in equation (16) of CEN TS 1992-4-4, § 6.2.2.3	k <sub>3</sub>	[-]	1,0	2,0	2,0	2,0
Characteristic resistance	R30 V <sub>Rk,cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
	R60 V <sub>Rk, cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
	R90 V <sub>Rk, cp,fi</sub>	[kN]	2,9	10,0	15,8	24,7
	R120 V <sub>Rk, cp,fi</sub>	[kN]	2,3	8,0	12,7	19,8

Concrete edge failure							
Eff. length of anchor under shear loading	l <sub>f</sub>	[mm]	48	60	72	86	
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16	

Design under fire exposure is performed according to the design method given in TR 020. Under fire exposure usually cracked concrete is assumed. The design equations are given in TR 020, Section 2.2.2.

TR 020 covers design for fire exposure from one side. For fire attack from more than one side the edge distance must be increased to  $c_{min} \ge 300$  mm and  $\ge 2 \cdot h_{ef}$ .

Mungo m1t- expansion anchor	
Design according to CEN/TS 1992-4 Characteristic shear resistance under fire exposure	Annex C8

Table 12: Characteristic values for resistance in case of seismic performance

category C1 acc. TR045 "Design of Metal anchor under Seismic A	tions"
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Anchor sizes			M8	M10	M12	M16
Tension load						
Steel failure						
Characteristic resistance	N <sub>Rk,s,s</sub> eis	[kN]	23,8	38,7	54,7	98,4
Partial safety factor <sup>1)</sup>	γ̃Ms,seis	[-]		1	,5	
Pull-out failure $N_{Rk,p,seis} = \Psi_c \times N^0$	Rk,p, <u>s</u> eis					
Characteristic resistance	N <sup>0</sup> <sub>Rk,p,seis</sub>	[kN ]	6	12	16	20
Partial safety factor <sup>1)</sup>	$\gamma$ Mp, seis	[-]		1	,5	
Shear loads						
Steel failure without lever arm						
Characteristic resistance	$V_{Rk,s,seis}$	[kN]	7,7	17,0	30,4	57,6
Partial safety factor <sup>1)</sup>	γ̃Ms, seis	[-]		1	,5	

 $<sup>^{1)}</sup>$  The recommended partial safety factors under seismic action ( $\gamma_{\text{M,seis}})$  are the same as for static loading

Mungo m1t- expansion anchor	
Design according to TR045 Characteristic resistance under seismic actions	Annex C9

Table 13: Characteristic values for resistance in case of seismic performance category C2 acc. TR045 "Design of Metal anchor under Seismic Actions"

Anchor sizes			M8	M10	M12	M16
Tension load						
Steel failure						
Characteristic resistance 2)	N <sub>Rk,s,s</sub> eis	[kN]	-	-	54,7	98,4
Partial safety factor <sup>3)</sup>	γMs,seis	[-]		1	,5	
Pull-out failure $N_{\underline{Rk},p,\underline{s}eis} = \Psi_c x$	$(N^0_{\underline{Rk},p,\underline{s}eis})$					
Characteristic resistance <sup>2)</sup>	N <sup>0</sup> <sub>Rk,p,seis</sub>	[kN]	-	1	11,8	20,0
Partial safety factor <sup>3)</sup>	γ̃Mp, seis	[-]		1	,5	
Displacement at DLS <sup>1) 2)</sup>	$\delta_{\text{N,sei (DSL)}}$	[mm]	-	ı	5,0	4,4
Displacement at DLS 1) 2)	$\delta_{\text{N,sei (ULS)}}$	[mm]	-	ı	20,4	17,8
Shear loads						
Steel failure without lever arm	1					
Characteristic resistance <sup>2)</sup>	$V_{Rk,s,seis}$	[kN]	-	-	19,3	31,2
Partial safety factor <sup>3)</sup>	γ̃Ms, seis	[-]	1,5			
Displacement at DLS 1) 2)	$\delta_{\text{V,sei (DSL)}}$	[mm]	-	-	7,0	7,0
Displacement at DLS <sup>1) 2)</sup>	$\delta_{\text{V,sei (ULS)}}$	[mm]	-	-	9,1	6,6

<sup>1)</sup> The listed displacements represent mean values.

Mungo m1t- expansion anchor	
Design according to TR045 Characteristic resistance under seismic actions	Annex C10

<sup>2)</sup> A smaller displacement may be required in the design provisions stated in section "Design of Anchorage", e.g. in the case of displacement sensitive fastenings or "rigid" supports. The characteristic resistance associated with such smaller displacement may be determined by linear interpolation or proportional reduction.

<sup>3)</sup> The recommended partial safety factors under seismic action ( $\gamma_{M,seis}$ ) are the same as for static loading.

Table 14: Displacements under tension loading

			M8	M10	M12	M16
Tension load in non-cracked co	ncrete C	20/25 [kN]	4,29	7,62	9,52	16,67
Dianlacement	$\delta_{\text{N0}}$	[mm]	0,1	0,1	0,1	0,1
Displacement	δ <sub>N</sub> ∞	[mm]	0,5	0,5	0,5	0,5
Tension load in non-cracked concrete C50/60 [kN]			6,64	11,91	14,76	25,83
Displacement	$\delta_{N0}$	[mm]	0,1	0,2	0,2	0,3
	δ <sub>N</sub> ∞	[mm]	0,5	0,5	0,5	0,5
Tension load in cracked concret	e C20/25	[kN]	2,86	5,71	7,62	9,52
Dianlacement	$\delta_{N0}$	[mm]	1,4	1,2	0,9	0,6
Displacement	δ <sub>N</sub> ∞	[mm]	1,4	1,2	1,3	0,6
Tension load in cracked concrete C50/60 [kN]			4,43	8,86	11,81	14,76
i ·	$\delta_{N0}$	[mm]	1,8	1,8	1,8	1,8
Displacement	δ <sub>N</sub> ∞	[mm]	1,8	1,8	1,8	1,8

Table 15: Displacements under shear loads

			M8	M10	M12	M16
Shear load in cracked and non-cracked concrete C20/25 to C50/60 [kN]		6,19	11,43	16,19	31,43	
Displacement	$\delta_{V0}$	[mm]	2,3	2,6	2,9	3,3
	δ <sub>V</sub> ∞	[mm]	3,4	3,9	4,3	4,9

Additional displacement due to anular gap between anchor and fixture is to be taken into account.

Mungo m1t- expansion anchor	
Design	Annex C11
Displacements	